# Stress analysis for a porous medium containing a cylindrical rigid inclusion 

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## Summary

In the present work a plane strain problem of the equilibrium theory of elastic materials with voids is studied. The problem of a rigid inclusion in an infinite body is investigated. The solution generalize analogous results in classical elasticity.

## Introduction

This paper concerns plane problems in the equilibrium theory of linear elastic materials with voids. This theory was formulated by Cowin and Nunziato [1] as a linearization of a nonlinear theory for elastic porous bodies. The linear theory deals with small changes from a reference configuration of porous body. The independent kinematic variables are the displacement field $u_{i}$ and the change in volume fraction $\psi$.

The intended application of the theory is to behavior of solid materials with small, distributed voids as geological materials and biological materials.

In this paper we study the problem of a rigid inclusion in an infinite body which is uniformly stretched along one axis. This problem is of great practical and technological importance and in the context of classical elasticity has been a subject of various studies (see, e.g. [2,3]). In Section 2 we present the basic equations of the equilibrium theory of elastic materials with voids and derive the equations of the plane strain problem for homogeneous and isotropic bodies. Section 3 concerns the problem of a cylindrical rigid inclusion. The solution is presented in a closed form and generalize analogous results in classical elasticity.

## Basic Equations

Throughout this section $B$ is a regular region of three-dimensional Euclidean space. We let $\partial B$ denote the boundary of $B$ and designate by $n$ the outward unit normal of $\partial B$. We assume that the region $B$ is occupied by a linearly elastic material with voids. The body is referred to a system of rectangular Cartesian axes $O x_{i}$. Let $u$ be the displacement field over $B$. The linear strain measure $e_{i j}$ is given by

$$
\begin{equation*}
e_{i j}=\frac{1}{2}\left(u_{i, j}+u_{j, i}\right) \tag{1}
\end{equation*}
$$

Let $t_{i j}$ be the stress tensor and let $h_{i}$ be the equilibrated stress vector. The components of surface traction $t_{i}$ and the equilibrated stress $h$ are given by

[^0]\[

$$
\begin{equation*}
t_{i}=t_{j i} n_{j}, \quad h=h_{i} n_{i}, \tag{2}
\end{equation*}
$$

\]

respectively. The equilibrium equations are

$$
\begin{equation*}
t_{j i, j}+f_{i}=0, \quad h_{i, i}-g+l=0 \tag{3}
\end{equation*}
$$

where $f_{i}$ are the components of body force, $g$ is the intrinsic equilibrated body force and $l$ is the extrinsic equilibrated body force.

In the case of centrosymmetric isotropic material the constitutive equations are

$$
\begin{equation*}
t_{i j}=\lambda e_{r r} \delta_{i j}+2 \mu e_{i j}+\beta \psi \delta_{i j}, \quad h_{i}=\alpha \psi,_{i}, \quad g=\beta e_{r r}+\zeta \psi \tag{4}
\end{equation*}
$$

where $\psi$ is the volume fraction function, $\delta_{i j}$ is Kronecker's delta, and $\lambda, \mu, \beta, \alpha$ and $\zeta$ are constitutive coefficients. We restrict our attention to homogeneous materials so that the constitutive coefficients are constants. We assume that the internal energy density is a positive definite form. This assumption implies that [1]

$$
\begin{equation*}
\mu>0, \alpha>0, \quad \zeta>0,2 \mu+3 \lambda>0, \quad(2 \mu+3 \lambda) \zeta>3 b^{2} \tag{5}
\end{equation*}
$$

We assume that the region $B$ refers to a right cylinder with the open cross section $\Sigma$ and the smooth lateral boundary $\Pi$. The rectangular Cartesian coordinate frame is supposed to be chosen in such a way that the $x_{3}$-axis is parallel to the generators of $B$.We denote by $L$ the boundary of $\Sigma$.
In what follows we are interested in a plane strain problem with the displacement vector and the volume fraction function being specified in cylindrical coordinates $(r, \theta, z)$ as follows:

$$
\begin{equation*}
u_{r}=u(r, \theta), \quad u_{\theta}=v(r, \theta), \quad u_{z}=0, \quad \psi=\varphi(r, \theta),(r, \theta) \in \Theta . \tag{6}
\end{equation*}
$$

The geometrical equations (1) become

$$
\begin{equation*}
\varepsilon_{r r}=\frac{\partial u}{\partial r}, \quad \varepsilon_{\theta \theta}=\frac{1}{r}\left(\frac{\partial v}{\partial \theta}+u\right), \quad \varepsilon_{r \theta}=\frac{1}{2}\left(\frac{1}{r} \frac{\partial u}{\partial \theta}+\frac{\partial v}{\partial r}-\frac{1}{r} v\right) . \tag{7}
\end{equation*}
$$

The equilibrium equations (2) take the form

$$
\begin{align*}
& \frac{\partial \tau_{r r}}{\partial r}+\frac{1}{r} \frac{\partial \tau_{r \theta}}{\partial \theta}+\frac{1}{r}\left(\tau_{r r}-\tau_{\theta \theta}\right)=0 \\
& \frac{\partial \tau_{r \theta}}{\partial r}+\frac{1}{r} \frac{\partial \tau_{\theta \theta}}{\partial \theta}+\frac{2}{r} \tau_{r \theta}=0 \\
& \frac{1}{r} \frac{\partial}{\partial r}\left(r \chi_{r}\right)+\frac{1}{r} \frac{\partial \chi_{\theta}}{\partial \theta}-\gamma=0 \tag{8}
\end{align*}
$$

The constitutive equations (4) can be written in the form

$$
\begin{align*}
& \tau_{r r}=(\lambda+2 \mu) \varepsilon_{r r}+\lambda \varepsilon_{\theta \theta}+\beta \varphi, \quad \tau_{\theta \theta}=\lambda \varepsilon_{r r}+(\lambda+2 \mu) \varepsilon_{\theta \theta}+\beta \varphi, \quad \tau_{r \theta}=2 \mu \varepsilon_{r \theta} \\
& \chi_{r}=\alpha \frac{\partial \varphi}{\partial r}, \quad \chi_{\theta}=\frac{1}{r} \alpha \frac{\partial \varphi}{\partial \theta}, \quad \gamma=\beta\left[\frac{1}{r} \frac{\partial}{\partial r}(r u)+\frac{1}{r} \frac{\partial v}{\partial \theta}\right]+\zeta \varphi \tag{9}
\end{align*}
$$

The plane strain problem consists in the finding of the functions $u, v$ and $\psi$ on $\Sigma$, which satisfy the Eqs.(7)-(9) and the boundary conditions.

## The problem of a rigid inclusion

In this section we study the problem of a rigid cylindrical inclusion in an infinite body which is uniformly stretched along the axis $O x_{1}$. We assume that the elastic body occupies the region $B=\left\{\left(x_{1}, x_{2}, x_{3}\right) \in R^{3}: x_{1}^{2}+x_{2}^{2}>a^{2}\right\}$, where $a$ is a positive constant. We assume that the region $\left\{\left(x_{1}, x_{2}, x_{3}\right) \in R^{3}: x_{1}^{2}+x_{2}^{2}<a^{2}\right\}$ is occupied by a rigid body. We consider the following boundary conditions:

$$
\begin{equation*}
u_{r}=0, \quad u_{\theta}=0, \quad \varphi=0, \text { on } r=a, \tag{10}
\end{equation*}
$$

and the conditions at infinity

$$
\begin{equation*}
\tau_{r r}=\frac{1}{2} P(1+\cos 2 \theta), \quad \tau_{\theta \theta}=\frac{1}{2} P(1-\cos 2 \theta), \quad \tau_{r \theta}=\tau_{\theta r}=-\frac{1}{2} P \sin 2 \theta, \quad \chi_{r}=\chi_{\theta}=0 \tag{11}
\end{equation*}
$$

where $P$ is a given constant. the body $B$ is in a state of plane strain parallel to the plane $x_{1} O x_{2}$ in the absents of body loads. We seek the solution in the form

$$
\begin{equation*}
u=F(r)+U(r) \cos 2 \theta, \quad v=V(r) \sin 2 \theta, \quad \varphi=G(r)+\Phi(r) \cos 2 \theta \tag{12}
\end{equation*}
$$

where $F, G, U, V$ and $\Phi$ are function only on $r$. It follows from (7), (12) and (9) that

$$
\begin{gather*}
\tau_{r r}=(\lambda+2 \mu) F^{\prime}+\frac{1}{r} \lambda F+\beta G+\left[(\lambda+2 \mu) U^{\prime}+\frac{1}{r} \lambda(U+2 V)+\beta \Phi\right] \cos 2 \theta, \\
\tau_{\theta \theta}=\lambda F^{\prime}+\frac{1}{r}(\lambda+2 \mu) F+\beta G+\left[\lambda U^{\prime}+\frac{1}{r}(\lambda+2 \mu)(U+2 V)+\beta \Phi\right] \cos 2 \theta, \\
\tau_{r \theta}=\tau_{\theta r}=\left[\mu V^{\prime}-\frac{1}{r} \mu(2 U+V)\right] \sin 2 \theta, \quad \chi_{r}=\alpha\left(G^{\prime}+\Phi^{\prime} \cos 2 \theta\right), \quad \chi_{\theta}=-\frac{2}{r} \alpha \sin 2 \theta,  \tag{13}\\
\gamma=\beta\left(F^{\prime}+\frac{1}{r} F\right)+\zeta G+\left\{\beta\left[U^{\prime}+\frac{1}{2}(U+2 V)\right]+\zeta \Phi\right\} \cos 2 \theta,
\end{gather*}
$$

where the prime denote derivation respect to $r$.If we substitute (13) in the equilibrium equations (8), we obtain the following equations:

$$
\begin{gather*}
(\lambda+2 \mu)\left(F^{\prime \prime}+\frac{1}{r} F^{\prime}-\frac{1}{r^{2}} F\right)+\beta G^{\prime}=0, \quad \alpha\left(G^{\prime \prime}+\frac{1}{r} G^{\prime}-\frac{\zeta}{\alpha} G\right)-\beta\left(F^{\prime}+\frac{1}{r} F\right)=0 \\
(\lambda+2 \mu)\left(r^{2} U^{\prime \prime}+r U^{\prime}\right)+(\mu+\lambda) r V^{\prime}+\beta r^{2} \Phi-(\lambda+6 \mu) U-2(\lambda+3 \mu) V=0 \\
\mu\left(r^{2} V^{\prime \prime}+r V^{\prime}\right)-2(\lambda+2 \mu) r U^{\prime}-2(\lambda+3 \mu) U-(4 \lambda+9 \mu) V-2 \beta r \Phi=0  \tag{14}\\
\alpha\left(r^{2} \Phi^{\prime \prime}+r \Phi^{\prime}-4 \Phi-\frac{\zeta}{\alpha} r^{2} \Phi\right)-\beta r^{2} U^{\prime}-\beta r(U+2 V)=0
\end{gather*}
$$

The first equation of (14) implies that

$$
\begin{equation*}
F^{\prime}+\frac{1}{r} F+\frac{\beta}{(2 \mu+\lambda)} G=C_{1} \tag{15}
\end{equation*}
$$

where $C_{1}$ is an arbitrary constant. In view of (15), the second equation of (14) can be written in the form

$$
\begin{equation*}
G^{\prime \prime}+\frac{1}{r} G^{\prime}-\xi^{2} G=\frac{\beta}{\alpha} C_{1}, \tag{16}
\end{equation*}
$$

where

$$
\begin{equation*}
\xi^{2}=\frac{1}{\alpha}\left(\zeta-\frac{\beta^{2}}{\lambda+2 \mu}\right) \tag{17}
\end{equation*}
$$

It follows from (5) that $\xi^{2}>0$. Since the function $G$ must be finite at infinity, the solution of Eq. (16) is

$$
\begin{equation*}
G=A_{1} K_{0}(\xi r)-\frac{\beta}{\xi^{2} \alpha} C_{1} \tag{18}
\end{equation*}
$$

where $I_{n}$ and $K_{n}$ are the modified Bessel functions of order $n, A_{1}$ is an arbitrary constant. It follows from (18) and (15) that

$$
\begin{equation*}
F=\frac{\zeta}{2 \xi^{2} \alpha} C_{1} r+\frac{1}{r} C_{2}+\frac{\beta A_{1}}{\xi(\lambda+2 \mu)} K_{1}(\xi r) \tag{19}
\end{equation*}
$$

where $C_{2}$ is an arbitrary constant. Now we introduce the independent variable $t$ through the relation $t=\ln r$, and denote $D=d / d t$. Then, Eqs.(14) ${ }_{3,4}$ can be written in the form

$$
\begin{align*}
& {\left[D^{2}-\left(1+4 c_{1}\right)\right] U+2\left[\left(1-c_{1}\right) D-\left(1+c_{1}\right)\right] V=-e^{t} c_{2} D \Phi} \\
& {\left[\left(1-c_{1}\right) D+\left(1+c_{1}\right)\right] U+\left[c_{1} D^{2}-\left(4+c_{1}\right)\right] V=2 e^{t} c_{2} \Phi} \tag{20}
\end{align*}
$$

where

$$
\begin{equation*}
c_{1}=\frac{\mu}{\lambda+2 \mu}, \quad c_{2}=\frac{\beta}{\lambda+2 \mu} . \tag{21}
\end{equation*}
$$

The general solution of the homogeneous system (20) which corresponds to a finite stress field at infinity is given by

$$
\begin{equation*}
U_{0}=b_{1} e^{-t}+B_{2} e^{-3 t}+B_{3} e^{t}, \quad V_{0}=-c_{1} B_{1} e^{-t}+B_{2} e^{-3 t}-B_{3} e^{t} \tag{22}
\end{equation*}
$$

where $B_{1}, B_{2}$ and $B_{3}$ are arbitrary constants. Particular solution of the system (20) can be seen to be

$$
\begin{equation*}
U^{*}=-\frac{1}{2} c_{2}\left(e^{t} S_{1}+e^{-3 t} S_{2}\right), \quad V^{*}=\frac{1}{2} c_{2}\left(e^{t} S_{1}-e^{-3 t} S_{2}\right) \tag{23}
\end{equation*}
$$

where

$$
\begin{equation*}
S_{1}(t)=\int_{t} \Phi(s) d s, \quad S_{2}(t)=\int_{t} e^{4 s} \Phi(s) d s \tag{24}
\end{equation*}
$$

With the help of (22) and (23) we obtain

$$
U=B_{1} r^{-1}+B_{2} r^{-3}+B_{3} r-\frac{1}{2} c_{2}\left[r \int_{r} x^{-1} \Phi(x) d x+r^{-3} \int_{r} x^{3} \Phi(x) d x\right]
$$

$$
\begin{equation*}
V=-c_{1} B_{1} r^{-1}+B_{2} r^{-3}-B_{3} r+\frac{1}{2} c_{2}\left[r \int_{r} x^{-1} \Phi(x) d x-r^{-3} \int_{r} x^{3} \Phi(x) d x\right] \tag{25}
\end{equation*}
$$

If we substitute $U$ and $V$ from (25) we obtain the equation

$$
\begin{equation*}
r^{2} \Phi^{\prime \prime}+r \Phi^{\prime}-\left(4+\xi^{2} r^{2}\right) \Phi=-\frac{2 c_{1} \beta B_{1}}{\alpha} \tag{26}
\end{equation*}
$$

The solution of Eq.(26) which generate finite stresses for $r \rightarrow \infty$ are given by

$$
\begin{equation*}
\Phi=A_{2} K_{2}(\xi r)+\frac{2}{\xi^{2} \alpha} c_{1} \beta B_{1} r^{-2} \tag{27}
\end{equation*}
$$

where $A_{2}$ is an arbitrary constant. If we substitute (27) into relations (25) we obtain

$$
\begin{align*}
U & =\frac{1}{r} B_{1}+\frac{1}{r^{3}} B_{2}+B_{3} r+\frac{1}{2 \xi} c_{2} A_{2}\left[K_{3}(\xi r)-K_{1}(\xi r)\right] \\
V & =-\frac{1}{r} c_{2} d B_{1}+\frac{1}{r^{3}} B_{2}-B_{3} r+\frac{1}{2 \xi} c_{2} A_{2}\left[K_{3}(\xi r)-K_{1}(\xi r)\right] \tag{28}
\end{align*}
$$

where $d=\zeta / \alpha \xi^{2}$. We introduce the notations

$$
\begin{equation*}
q_{1}=1-2 c_{1} d, q_{2}=2-c_{1} d, \quad Q=\frac{\zeta(\lambda+\mu)-\beta^{2}}{\zeta(\lambda+\mu)-\beta^{2}}, \quad K=2 \zeta(\lambda+\mu)-\beta^{2} \tag{29}
\end{equation*}
$$

It follows from (9),(18),(19) and (27)-(29) that

$$
\begin{align*}
t_{r r}= & \frac{K}{2 \alpha \xi^{2}} c_{1}-2 \mu r^{-2} c_{2}-2 \mu\left\{2 Q r^{-2} B_{1}+3 r^{-4} B_{2}-B_{3}+\frac{1}{4 \xi} c_{2} A_{3}\left[6 r^{-1} K_{3}(\xi r)-\right.\right. \\
& \left.\left.\xi K_{2}(\xi r)+\xi K_{0}(\xi r)\right]\right\} \cos 2 \theta \\
t_{\theta \theta}= & \frac{K}{2 \alpha \xi^{2}} c_{1}+2 \mu r^{-2} c_{2}+2 \mu c_{2} A_{1}\left[K_{0}(\xi r)+\frac{1}{\xi r} K_{1}(\xi r)\right]+2 \mu\left\{3 r^{-4} B_{2}-B_{3}+\right. \\
& \left.\frac{1}{4 \xi} c_{2} A_{3}\left[3 \xi K_{2}(\xi r)+\xi K_{0}(\xi r)-6 r^{-1} \xi K_{3}(\xi r)\right]\right\} \cos 2 \theta, \\
t_{r \theta}= & 2 \mu\left\{-\frac{Q}{2} B_{1} r^{-2}-3 r^{-4} B_{2}-B_{3}-\frac{1}{2 \xi} c_{2} A_{3} r^{-1}\left[3 K_{3}(\xi r)+K_{1}(\xi r)\right]\right\} \sin 2 \theta, \\
h_{r}= & -\alpha \xi A_{1} K_{1}(\xi r)-\left\{\alpha A_{3}\left[\xi K_{1}(\xi r)+2 r^{-1} K_{1}(\xi r)\right]+4 B_{1} r^{-3} c_{1} \frac{\beta}{\xi^{2}}\right\} \cos 2 \theta, \\
& h_{\theta}=-\left\{2 \alpha r^{-1} A_{3} K_{3}(\xi r)+4 B_{1} r^{-3} c_{1} \frac{\beta}{\xi^{2}}\right\} \sin 2 \theta . \tag{30}
\end{align*}
$$

On the basis of (30) the conditions at infinity (11) reduce to

$$
\begin{equation*}
B_{3}=\frac{1}{4 \mu} P, \quad C_{1}=\frac{\alpha \xi^{2}}{K} P \tag{31}
\end{equation*}
$$

We note that the restrictions (5) imply that $K>0$. With help of (18), (19),(27) and (28) we find that the conditions (6) can be written in the form
$A_{1}=\frac{\beta}{K} P\left[K_{0}(\xi a)\right]^{-1}, C_{2}=-\frac{\zeta a^{2}}{2 K} P-\frac{c_{2} a}{\xi} A_{1} K_{1}(\xi a), A_{2}=-\frac{2}{\alpha \xi^{2} a^{2}} c_{1} \beta B_{1}\left[K_{2}(\xi a)\right]^{-1}$,
$\left[a^{2}-\frac{c_{1} c_{2} \beta a}{a \xi^{3}} L(\xi a)\right] B_{1}+B_{2}=-\frac{P a^{4}}{4 \mu}, \quad\left[-c_{2} d a^{2}-\frac{c_{1} c_{2} \beta a}{a \xi^{3}} L(\xi a)\right] B_{1}+B_{2}=\frac{P a^{4}}{4 \mu}$,
where

$$
\begin{equation*}
L(z)=\left[K_{3}(z)+K_{1}(z)\right]\left[K_{2}(z)\right]^{-1} . \tag{33}
\end{equation*}
$$

From (32) we obtain

$$
\begin{equation*}
B_{1}=-\frac{P a^{2}}{2 \mu\left(1+c_{2} d\right)}, \quad B_{2}=\frac{P a^{2}}{4 \mu\left(1+c_{2} d\right)}\left[\left(1-c_{2} d\right) a^{2}-\frac{2 c_{1} c_{2} \beta a}{a \xi^{3}} L(\xi a)\right] . \tag{34}
\end{equation*}
$$

The solution of the problem has the form (12) where the constants $A_{\alpha}, B_{i}$ and $C_{\alpha}$ are given by (31), (32) and (34).The stress tensor and microstress vector can be determined from the relations (30).In particular, the values of $t_{r r}$ and $t_{r \theta}$ on the boundary of the inclusion have the form

$$
\begin{align*}
& t_{r r}=\frac{1}{2} P+\frac{\mu}{K} \zeta P\left[\frac{2 \beta c_{2}}{\xi \zeta a} \frac{K_{1}(\xi a)}{K_{0}(\xi a)}+1\right]-\frac{P a^{2}}{1+c_{2} d}\left[\left(-2 Q+1-2 c_{2} d\right) a^{2}-\frac{4 c_{1} c_{2} \beta K_{1}(\xi a)}{\alpha \xi^{3} a^{3} K_{2}(\xi a)}\right] \cos 2 \theta, \\
& t_{r \theta}=-\frac{P a^{2}}{1+c_{2} d}\left[\left(-Q+\frac{1}{2}-\frac{5}{2} c_{2} d\right) a^{-2}-\frac{2 c_{1} c_{2} \beta K_{1}(\xi a)}{\alpha \xi^{2} a^{3} K_{2}(\xi a)}\right] \sin 2 \theta . \tag{35}
\end{align*}
$$

The problem of a rigid inclusion in an elastic medium has been investigated also in the context of non-classical theories (see e.g. [4-5]).

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